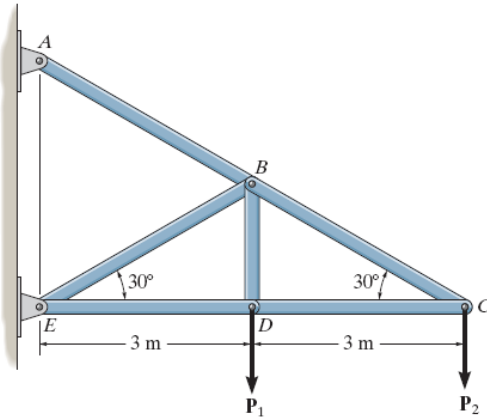


HW 8 SOLUTIONS

6-6. Determine the force in each member of the truss and state if the members are in tension or compression. Set $P_1 = 2 \text{ kN}$ and $P_2 = 1.5 \text{ kN}$.



Method of Joints : In this case, the support reactions are not required for determining the member forces.

Joint C

$$+\uparrow \Sigma F_y = 0; \quad F_{CB} \sin 30^\circ - 1.5 = 0$$

$$F_{CB} = 3.00 \text{ kN (T)} \quad \text{Ans}$$

$$+\rightarrow \Sigma F_x = 0; \quad F_{CD} - 3.00 \cos 30^\circ = 0$$

$$F_{CD} = 2.598 \text{ kN (C)} = 2.60 \text{ kN (C)} \quad \text{Ans}$$

Joint D

$$+\rightarrow \Sigma F_x = 0; \quad F_{DE} - 2.598 = 0 \quad F_{DE} = 2.60 \text{ kN (C)} \quad \text{Ans}$$

$$+\uparrow \Sigma F_y = 0; \quad F_{DB} - 2 = 0 \quad F_{DB} = 2.00 \text{ kN (T)} \quad \text{Ans}$$

Joint B

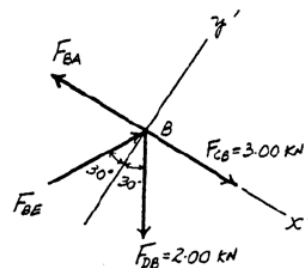
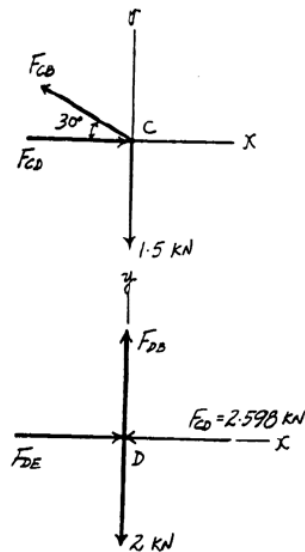
$$+\nearrow \Sigma F_{y'} = 0; \quad F_{BE} \cos 30^\circ - 2.00 \cos 30^\circ = 0$$

$$F_{BE} = 2.00 \text{ kN (C)} \quad \text{Ans}$$

$$+\searrow \Sigma F_{x'} = 0; \quad (2.00 + 2.00) \sin 30^\circ + 3.00 - F_{BA} = 0$$

$$F_{BA} = 5.00 \text{ kN (T)} \quad \text{Ans}$$

Note : The support reactions at support A and E can be determined by analyzing Joints A and E respectively using the results obtained above.



*6-8. Determine the force in each member of the truss, and state if the members are in tension or compression. Set $P = 800 \text{ lb}$.

Method of Joints: We will analyze the equilibrium of the joints in the following sequence:

$A \rightarrow F \rightarrow E \rightarrow B \rightarrow C$.

Joint A: From the free-body diagram in Fig. a,

$$+\uparrow \Sigma F_y = 0; \quad F_{AF} \sin 45^\circ - 800 = 0$$

$$F_{AF} = 1131.37 \text{ lb} = 1131 \text{ lb (T)} \quad \text{Ans.}$$

$$\rightarrow \Sigma F_x = 0 \quad 1131.37 \cos 45^\circ - F_{AB} = 0$$

$$F_{AB} = 800 \text{ lb (C)} \quad \text{Ans.}$$

Joint F: From the free-body diagram in Fig. b,

$$+\uparrow \Sigma F_y = 0; \quad F_{FB} \cos 45^\circ - 1131.37 \cos 45^\circ - 500 = 0$$

$$F_{FB} = 1838.48 \text{ lb} = 1838 \text{ lb (C)} \quad \text{Ans.}$$

$$\rightarrow \Sigma F_x = 0; \quad F_{FE} - 1838.48 \sin 45^\circ - 1131.37 \sin 45^\circ = 0$$

$$F_{FE} = 2100 \text{ lb (T)} \quad \text{Ans.}$$

Joint E: From the free-body diagram in Fig. c,

$$\rightarrow \Sigma F_x = 0; \quad F_{ED} - 2100 = 0$$

$$F_{ED} = 2100 \text{ lb (T)} \quad \text{Ans.}$$

$$+\uparrow \Sigma F_y = 0; \quad F_{EB} = 0 \quad \text{Ans.}$$

Joint B: From the free-body diagram in Fig. d,

$$+\uparrow \Sigma F_y = 0; \quad F_{BD} \sin 45^\circ - 1838.48 \sin 45^\circ = 0$$

$$F_{BD} = 1838.48 \text{ lb} = 1838 \text{ lb (T)} \quad \text{Ans.}$$

$$\rightarrow \Sigma F_x = 0; \quad 800 + 1838.48 \cos 45^\circ + 1838.48 \cos 45^\circ - F_{BC} = 0$$

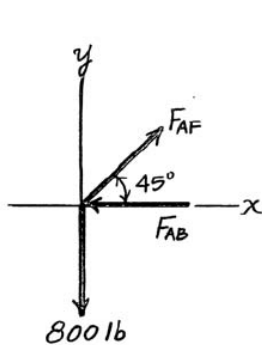
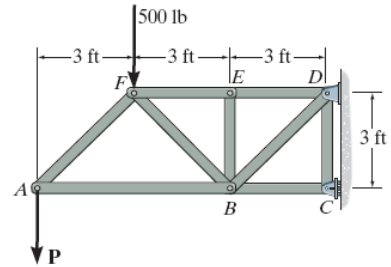
$$F_{BC} = 3400 \text{ lb (C)} \quad \text{Ans.}$$

Joint C: From the free-body diagram in Fig. e,

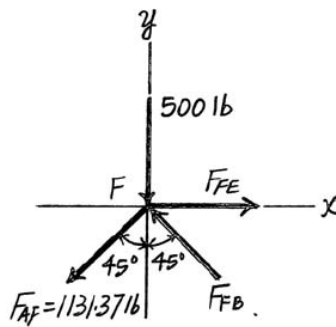
$$+\uparrow \Sigma F_y = 0; \quad F_{CD} = 0 \quad \text{Ans.}$$

$$\rightarrow \Sigma F_x = 0; \quad 3400 - N_C = 0$$

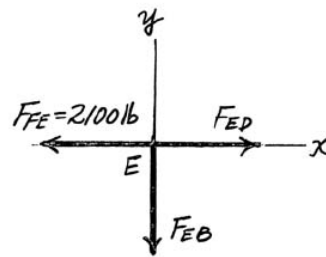
$$N_C = 3400 \text{ lb}$$



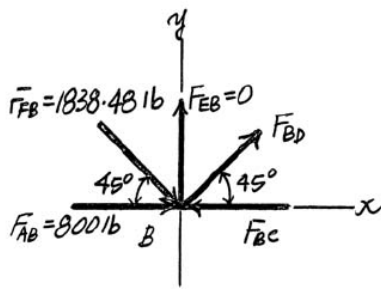
(a)



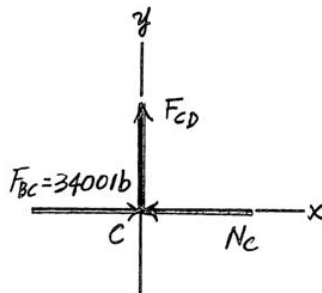
(b)



(c)



(d)



(e)

6-14. Determine the force in each member of the truss, and state if the members are in tension or compression. Set $P = 2500$ lb.

Support Reactions: Applying the moment equation of equilibrium about point A to the free-body diagram of the truss, Fig. a ,

$$\begin{aligned} +\circlearrowleft \Sigma M_A &= 0; & N_B(8+8) - 1200(8+8) - 2500(8) &= 0 \\ & & N_B &= 2450 \text{ lb} \end{aligned}$$

Method of Joints: We will begin by analyzing the equilibrium of joint B , and then that of joints C and G .

Joint B : From the free-body diagram in Fig. b ,

$$\begin{aligned} +\rightarrow \Sigma F_x &= 0; & F_{BG} &= 0 & \text{Ans.} \\ +\uparrow \Sigma F_y &= 0; & 2450 - F_{BC} &= 0 \\ & & F_{BC} &= 2450 \text{ lb (C)} & \text{Ans.} \end{aligned}$$

Joint C : From the free-body diagram in Fig. c ,

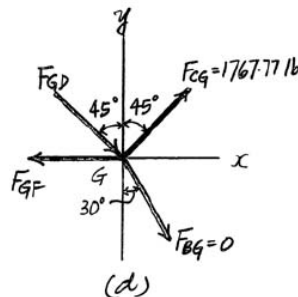
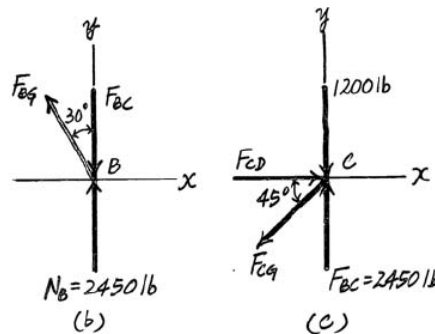
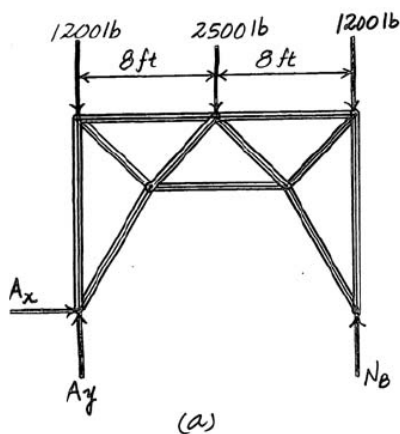
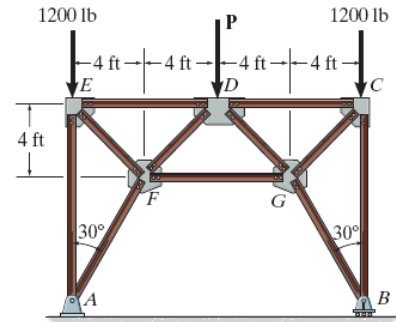
$$\begin{aligned} +\uparrow \Sigma F_y &= 0; & 2450 - 1200 - F_{CG} \sin 45^\circ &= 0 \\ & & F_{CG} &= 1767.77 \text{ lb} = 1768 \text{ lb (T)} & \text{Ans.} \\ +\rightarrow \Sigma F_x &= 0; & F_{CD} - 1767.77 \cos 45^\circ &= 0 \\ & & F_{CD} &= 1250 \text{ lb (C)} & \text{Ans.} \end{aligned}$$

Joint G : From the free-body diagram in Fig. d ,

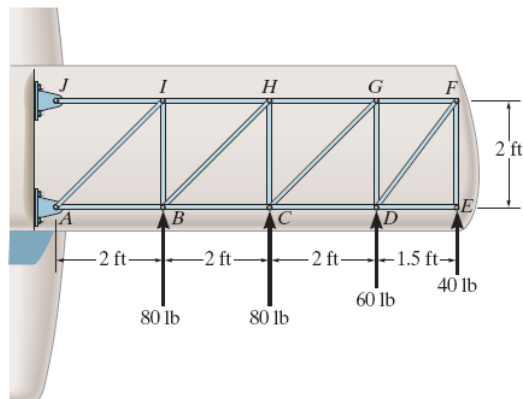
$$\begin{aligned} +\uparrow \Sigma F_y &= 0; & 1767.77 \cos 45^\circ - F_{GD} \cos 45^\circ &= 0 \\ & & F_{GD} &= 1767.77 \text{ lb} = 1768 \text{ lb (C)} & \text{Ans.} \\ +\rightarrow \Sigma F_x &= 0; & 1767.77 \sin 45^\circ + 1767.77 \sin 45^\circ - F_{GF} &= 0 \\ & & F_{GF} &= 2500 \text{ lb (T)} & \text{Ans.} \end{aligned}$$

Due to the symmetry of the system and the loading,

$$\begin{aligned} F_{AE} &= F_{BC} = 2450 \text{ lb (C)} & \text{Ans.} \\ F_{AF} &= F_{BG} = 0 & \text{Ans.} \\ F_{ED} &= F_{CD} = 1250 \text{ lb (C)} & \text{Ans.} \\ F_{EF} &= F_{CG} = 1767.77 \text{ lb} = 1768 \text{ lb (T)} & \text{Ans.} \\ F_{FD} &= F_{GD} = 1767.77 \text{ lb} = 1768 \text{ lb (C)} & \text{Ans.} \end{aligned}$$



6-31. The internal drag truss for the wing of a light airplane is subjected to the forces shown. Determine the force in members BC , BH , and HC , and state if the members are in tension or compression.



$$+\uparrow \Sigma F_y = 0; \quad 180 - F_{BH} \sin 45^\circ = 0$$

$$F_{BH} = 255 \text{ lb (T)} \quad \text{Ans}$$

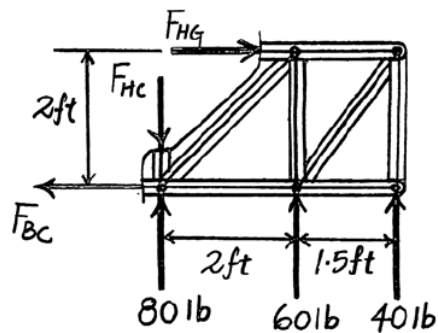
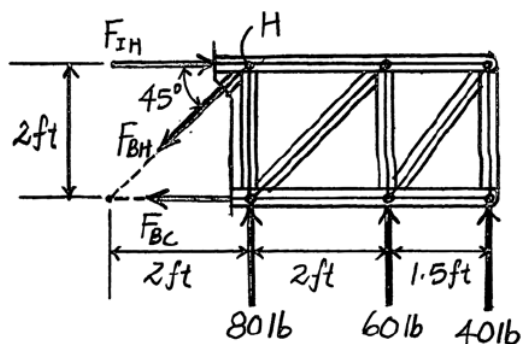
$$+\circlearrowleft \Sigma M_H = 0; \quad -F_{BC}(2) + 60(2) + 40(3.5) = 0$$

$$F_{BC} = 130 \text{ lb (T)} \quad \text{Ans}$$

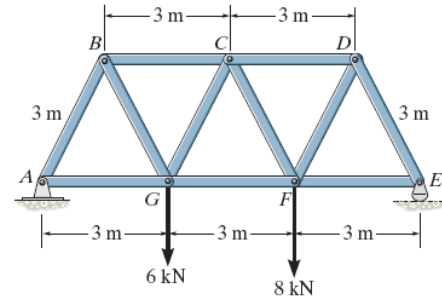
Section 2 :

$$+\uparrow \Sigma F_y = 0; \quad 80 + 60 + 40 - F_{HC} = 0$$

$$F_{HC} = 180 \text{ lb (C)} \quad \text{Ans}$$



*6-36. Determine the force in members BC , CG , and GF of the Warren truss. Indicate if the members are in tension or compression.



Support Reactions :

$$(+\Sigma M_E = 0; \quad 6(6) + 8(3) - A_y(9) = 0 \quad A_y = 6.667 \text{ kN}$$

$$+\Sigma F_x = 0; \quad A_x = 0$$

Method of Sections :

$$(+\Sigma M_C = 0; \quad F_{GF}(3 \sin 60^\circ) + 6(1.5) - 6.667(4.5) = 0$$

$$F_{GF} = 8.08 \text{ kN (T)}$$

Ans

$$(+\Sigma M_G = 0; \quad F_{BC}(3 \sin 60^\circ) - 6.667(3) = 0$$

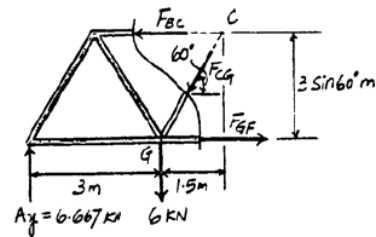
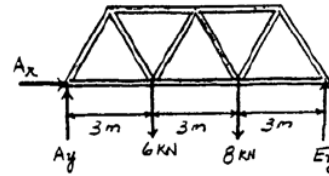
$$F_{BC} = 7.70 \text{ kN (C)}$$

Ans

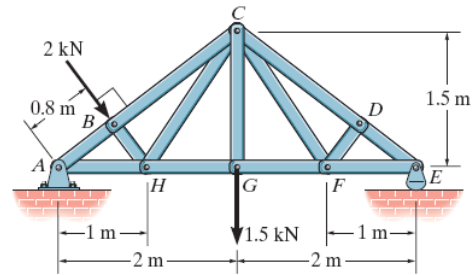
$$+\uparrow \Sigma F_y = 0; \quad 6.667 - 6 - F_{CG} \sin 60^\circ = 0$$

$$F_{CG} = 0.770 \text{ kN (C)}$$

Ans



6-46. Determine the force developed in members BC and CH of the roof truss and state if the members are in tension or compression.



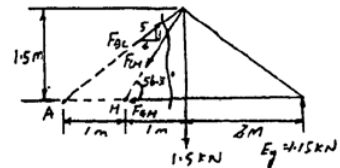
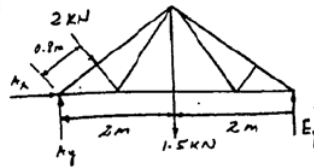
$$\sum \mathcal{M}_A = 0; \quad E_y(4) - 2(0.8) - 1.5(2) = 0 \quad E_y = 1.15 \text{ kN}$$

$$\sum \mathcal{M}_H = 0; \quad 1.15(3) - 1.5(1) - \frac{3}{5}F_{BC}(1) = 0$$

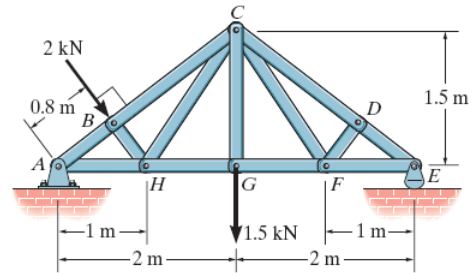
$$F_{BC} = 3.25 \text{ kN (C)} \quad \text{Ans}$$

$$\sum \mathcal{M}_A = 0; \quad 1.15(4) - 1.5(2) - F_{CH} \sin 56.31^\circ(1) = 0$$

$$F_{CH} = 1.92 \text{ kN (T)} \quad \text{An}$$



6-47. Determine the force in members CD and GF of the truss and state if the members are in tension or compression. Also indicate all zero-force members.



Entire truss :

$$\sum M_A = 0; \quad -2(0.8) - 1.5(2) + E_y(4) = 0$$

$$E_y = 1.15 \text{ kN}$$

Section :

$$\sum M_F = 0; \quad 1.15(1) - F_{CD} \sin 36.87^\circ(1) = 0$$

$$F_{CD} = 1.92 \text{ kN (C)} \quad \text{Ans}$$

$$\sum M_C = 0; \quad -F_{GF}(1.5) + 1.15(2) = 0$$

$$F_{GF} = 1.53 \text{ kN (T)} \quad \text{Ans}$$

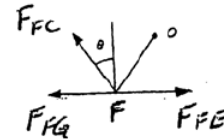
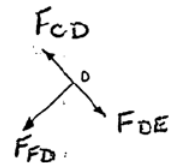
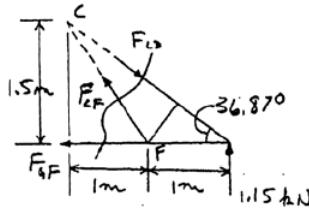
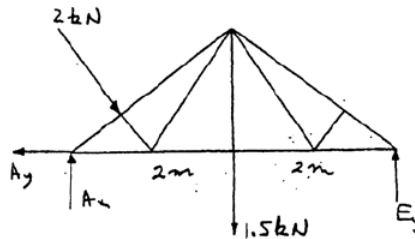
Joint D :

$$\sum F_y = 0; \quad F_{FD} = 0 \quad \text{Ans}$$

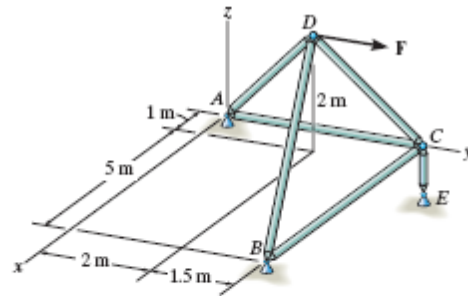
Joint F :

$$\sum F_y = 0; \quad F_{FC} \cos \theta = 0$$

$$F_{FC} = 0 \quad \text{Ans}$$



*6-56. Determine the force in each member of the space truss and state if the members are in tension or compression. The truss is supported by ball-and-socket joints at A, B, and E. Set $\mathbf{F} = \{800\mathbf{j}\}$ N. *Hint:* The support reaction at E acts along member EC. Why?



Joint D:

$$\Sigma F_x = 0; \quad -\frac{1}{3}F_{AD} + \frac{5}{\sqrt{31.25}}F_{BD} + \frac{1}{\sqrt{7.25}}F_{CD} = 0$$

$$\Sigma F_y = 0; \quad -\frac{2}{3}F_{AD} + \frac{1.5}{\sqrt{31.25}}F_{BD} - \frac{1.5}{\sqrt{7.25}}F_{CD} + 800 = 0$$

$$\Sigma F_z = 0; \quad -\frac{2}{3}F_{AD} - \frac{2}{\sqrt{31.25}}F_{BD} + \frac{2}{\sqrt{7.25}}F_{CD} = 0$$

$$F_{AD} = 686 \text{ N (T)} \quad \text{Ans}$$

$$F_{BD} = 0 \quad \text{Ans}$$

$$F_{CD} = 615.4 = 615 \text{ N (C)} \quad \text{Ans}$$

Joint C:

$$\Sigma F_x = 0; \quad F_{BC} - \frac{1}{\sqrt{7.25}}(615.4) = 0$$

$$F_{BC} = 229 \text{ N (T)} \quad \text{Ans}$$

$$\Sigma F_y = 0; \quad \frac{1.5}{\sqrt{7.25}}(615.4) - F_{AC} = 0$$

$$F_{AC} = 343 \text{ N (T)} \quad \text{Ans}$$

$$\Sigma F_z = 0; \quad F_{EC} - \frac{2}{\sqrt{7.25}}(615.4) = 0$$

$$F_{EC} = 457 \text{ N (C)} \quad \text{Ans}$$

